



*Development and testing of
improved physically based
streambank erosion and
sediment routing routines in
SWAT*

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Outline

- ✚ Channel Erosion
- ✚ Sediment Routing
 - ✚ In SWAT2000 and SWAT2005
- ✚ Physically based approach
 - ✚ Erosion
 - ✚ Transportation
 - ✚ Deposition
- ✚ Field monitoring and model results





Channel Erosion

✦ Channel erosion

- ✦ Can account for as much as 85% of total sediment yield of a watershed



Predicted loss in 3 km channel erosion = 1000 years of sheet and rill erosion at pre-conservation agriculture rates



Channel Erosion

✚ Three major processes

✚ Subaerial processes

- Climate
- Alternate wet and dry cycles
- Freeze/Thaw cycles
- Cracking

✚ Fluvial erosion (Hydraulic Erosion)

- Removal of particles by streamflow

✚ Bank Failure

- Caused due to slope instability





SWAT2000 and 2005

✦ Simplified Bagnold stream power equation

$$conc_{sed,mx} = spcon \times v_{ch}^{sp_{exp}}$$

$$sed_{deg} = (conc_{sed,mx} - conc_{sed,ch}) V_{ch} K_{ch} C_{ch}$$

✦ Channel erosion

- ✦ limited only by the stream power or transport capacity
- ✦ but not by limits on sediment supply from the actual erosion process



SWAT2000 and 2005

- ✚ No particle size distribution of eroded sediment
 - ✚ No bedload
 - Hence, TSS calculated from sediment yield is often high and not directly comparable with observations





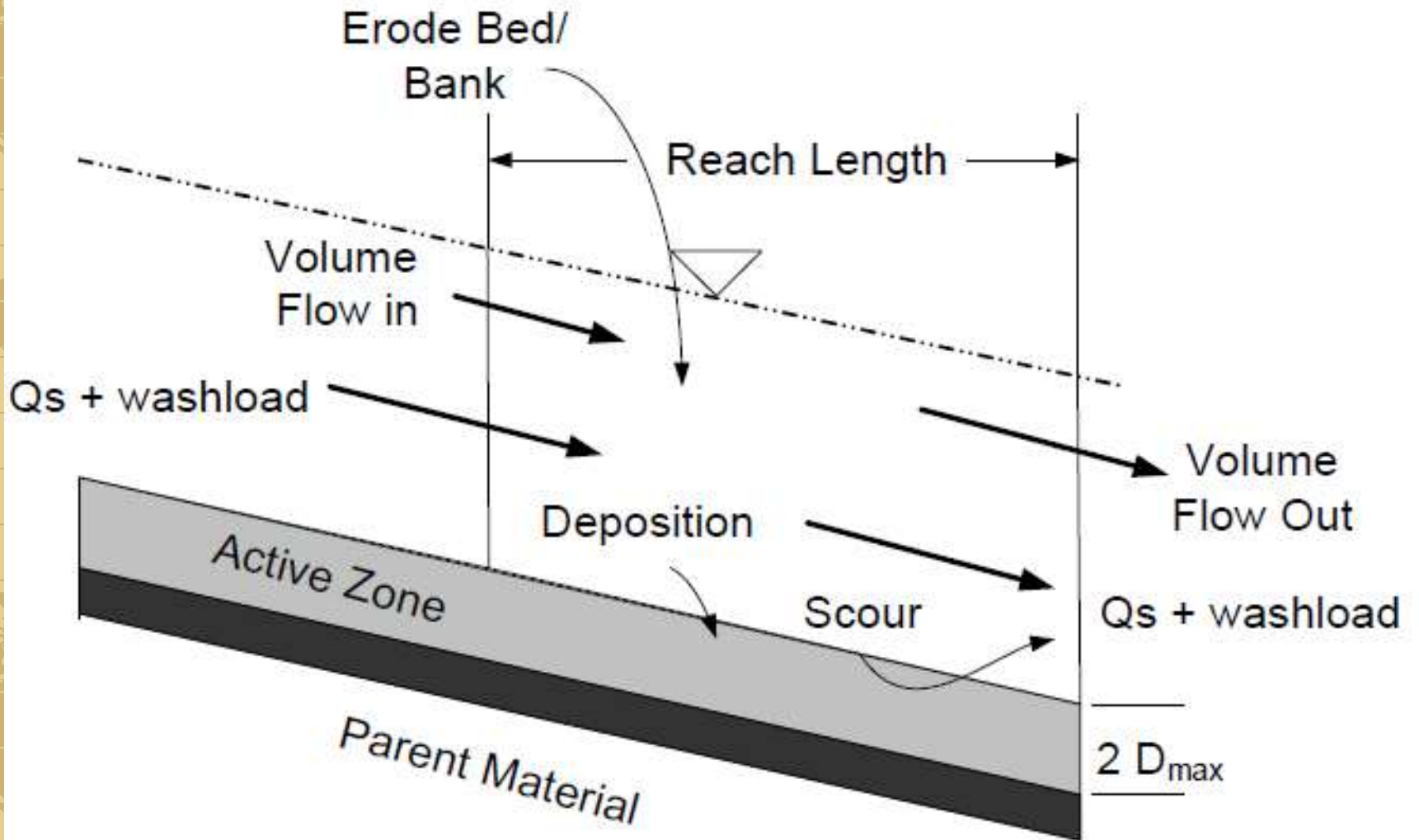
Organic nutrient load

- ✦ Are we missing to quantify a significant organic nutrient load from stream bank and attributing the nutrient loads only to overland?

- ✦ Cedar Creek, Texas

- 8% of orgN and
- 15% of orgP from channel erosion
- Channel erosion – 35% of total sediment yield

- ✦ Hence, accurate quantification of channel erosion is very important





Complex Process: Simplify





Fluvial Erosion Process

- ✦ For the erosion to occur
 - ✦ There should be enough shear stress exerted by the flowing water on stream bank and stream bed to dislodge the sediments
 - ✦ The channel should have enough stream power to carry the eroded sediments (overland+channel)
 - ✦ Deposition will occur if the sediment transport capacity is low





Wash-load particle size distribution

- Sediment yield from overland (MUSLE) is partitioned using the approach used in CREAMS

$$PSA = (SAN)(1. - CLA)^{2.4}$$

$$PSI = 0.13SIL$$

$$PCL = 0.20CLA$$

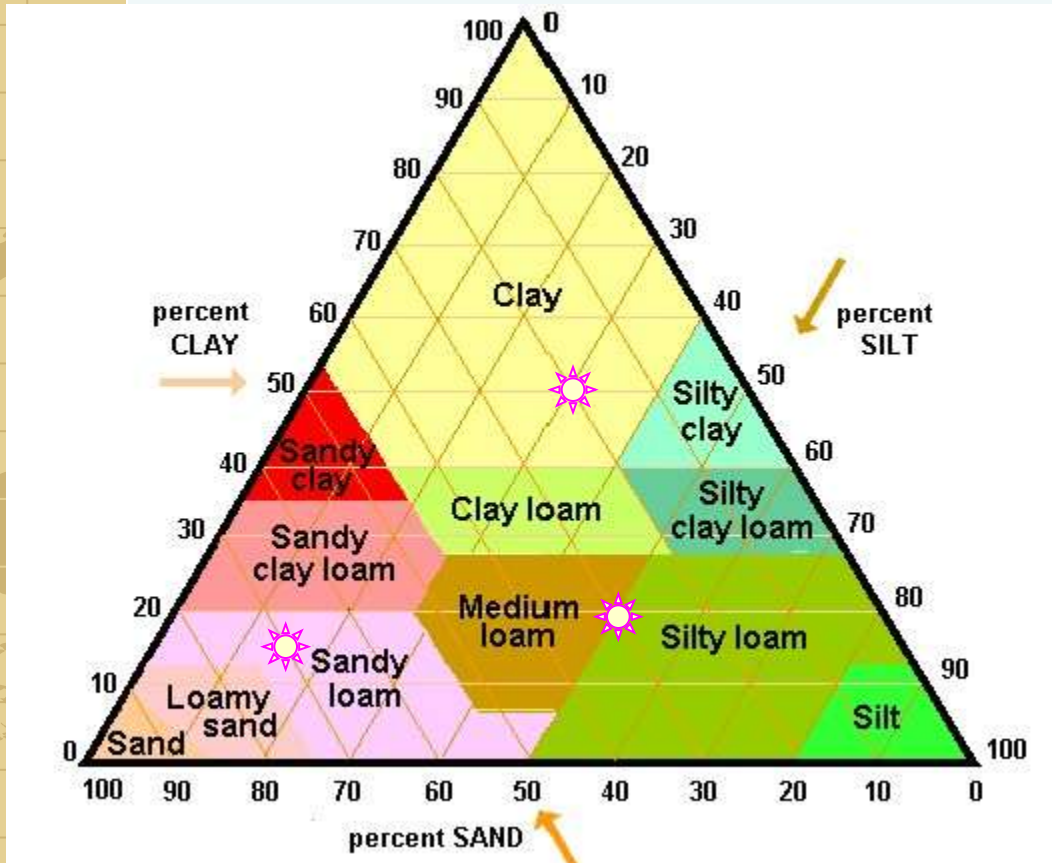
$$SAG = \begin{cases} 2.0CLA & \text{for } CLA < 0.25 \\ 0.28(CLA - 0.25) + 0.5 & \text{for } CLA \geq 0.25 \text{ } CLA \leq 0.5 \\ 0.57 & \text{for } CLA > 0.5 \end{cases}$$

$$LAG = 1.0 - PSA - PSI - PCL - SAG$$



Stream bank/bed erosion load particle distribution

☉ Channel bank and bed D50





Shear Stress

- ✦ Critical shear stress (τ_c)
 - ✦ Soil parameter that governs erosion
- ✦ Erosion based on excess shear stress:

$$\xi_{bank} = k_{d,bank} \cdot (\tau_{e,bank} - \tau_{c,bank})^a \cdot 10^{-6}$$

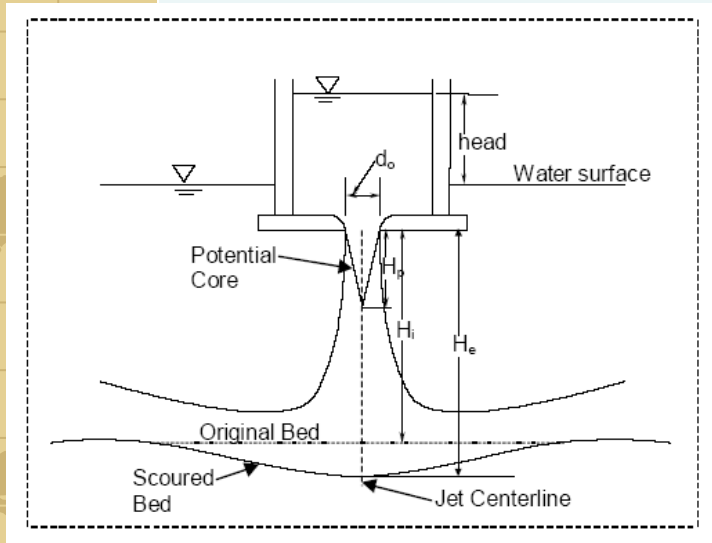
$$\xi_{bed} = k_{d,bed} \cdot (\tau_{e,bed} - \tau_{c,bed})^a \cdot 10^{-6}$$

- ✦ where ξ – erosion rates of the bank and bed (m/s), k_d – erodibility coefficient of bank and bed ($\text{cm}^3/\text{N}\cdot\text{s}$) and τ_c – Critical shear stress acting on bank and bed (N/m^2).



Critical Shear Stress and Erodibility Coefficient

Submerged Jet Test (Hanson and Cook, 1997; Hanson and Simon, 2001)



Hanson and Simon, 2001

- Erodibility calculated as a function of jet index

$$k = 0.003e^{385J_i}$$

- J_i – Jet index (depth of scour hole made by)

$$\tau_c = 0.16(I_w)^{0.84}$$

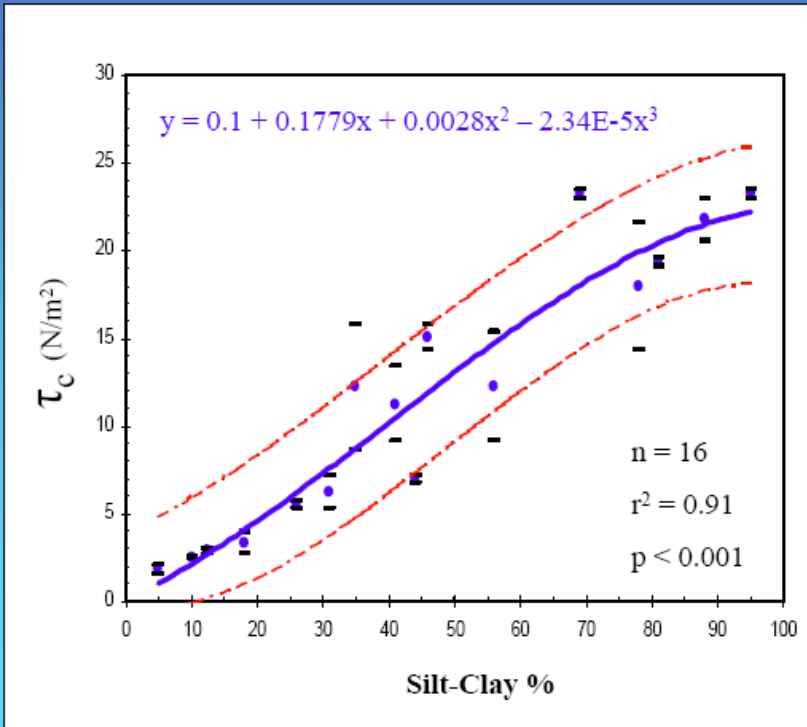
- I_w – Plasticity Index



Empirical Equation for τ_c

Critical Shear Estimates

Soil Composition



Vegetation

Type / Density	τ_c coefficient
None	1
Ivy / Sparse	1.5
Ivy / Dense	2.5
Privet / Sparse	5.4
Privet / Dense	19.2

Range mostly between:
 0 and 100 N/m²
 But could go as High as 400 N/m²

Julian and Torres, 2001





Shear Stress

- Effective shear stress based on channel hydraulics: (Eaton and Millar, 2004)

$$\frac{\tau_{e, bank}}{\gamma \cdot depth \cdot slp_{ch}} = \frac{SF_{bank}}{100} \left(\frac{(W + P_{bed}) \cdot \sin \theta}{4 \cdot depth} \right)$$

$$\frac{\tau_{e, bed}}{\gamma_w \cdot depth \cdot slp_{ch}} = \left(1 - \frac{SF_{bank}}{100} \right) \left(\frac{W}{2 \cdot P_{bed}} + 0.5 \right)$$

$$\log SF_{bank} = -1.4026 \cdot \log \left(\frac{P_{bed}}{P_{bank}} + 1.5 \right) + 2.247$$



Empirical Equation for K_d

- Erodibility Coefficient, K_d : (Temple and Hanson, 1994; Zhu et al. 2006)

$$K_d = 0.0034 \cdot \exp\left(\frac{0.0176}{M_e}\right)$$

$$M_e = \frac{((s-1) \cdot 9.8 \cdot D_{50})^{0.5}}{(s-1)^3 \cdot C}$$

$$C = 4.14 \cdot (Clay\%)^{-0.91}$$

Where s is relative density of sediment

Range mostly between 0 and 0.01 cm³/N-s but could go
As high as 3.75 cm³/N-s for highly erodible material



Stream Power/Transport Capacity

⊕ Four new transport equations

⊕ Simplified Bagnold Equation

- Silt type bed material

$$CONC_{sed, mx} = spcon \times v_{ch}^{sp_{exp}}$$

⊕ Kodatie model

- Silt to gravel size bed materials

⊕ Molinas and Wu model

- Large sand bed rivers

⊕ Yangs sand and gravel model

- Sand and gravel bed material



Kodatie Model

Kodatie (2000)

$$conc_{sed, ch.mx} = \left(\frac{a \cdot v_{ch}^b \cdot y^c \cdot S^d}{Q_{in}} \right) \cdot \left(\frac{W + W_{btm}}{2} \right)$$

a, b, c and d coefficients depend on D_{50}

Table 7:2-2. Regression coefficients for Kodatie equation

	a	b	c	d
Silt-bed rivers ($D_{50}^* \leq 0.05$ mm)	281.4	2.622	0.182	0
Very fine to fine-bed river (0.05 mm < $D_{50} \leq 0.25$ mm)	2,829.6	3.646	0.406	0.412
Medium to very coarse sand-bed rivers (0.25 mm < $D_{50} \leq 2$ mm)	2,123.4	3.300	0.468	0.613
Gravel-bed rivers ($D_{50} > 2$ mm)	431,884.8	1.000	1.000	2.000

* D_{50} – median bank/bed-sediment size



Molinas and Wu Model

✚ Molinas and Wu (2001):

$$C_w = \frac{1430 \cdot (0.86 + \sqrt{\psi}) \cdot \psi^{1.5}}{0.016 + \psi} \cdot 10^{-6}$$

$$\psi = \frac{v_{ch}^3}{(S_g - 1) \cdot g \cdot depth \cdot \omega_{50} \cdot \left[\log_{10} \left(\frac{depth}{D_{50}} \right) \right]^2}$$

$$\omega_{50} = \frac{411 \cdot D_{50}^2}{3600}$$

$$CONC_{sed, ch.mx} = \frac{C_w}{C_w + (1 - C_w) \cdot S_g} \cdot S_g$$



Yang's Sand and Gravel Model

- ✦ Sand equation: (D_{50} less than 2mm):

$$\log C_w = 5.435 - 0.286 \log \frac{\omega_{50} D_{50}}{\nu} - 0.457 \log \frac{V_*}{\omega_{50}} + \left(1.799 - 0.409 \log \frac{\omega_{50} D_{50}}{\nu} - 0.314 \log \frac{V_*}{\omega_{50}} \right) \log \left(\frac{v_{ch} S}{\omega_{50}} - \frac{V_{cr} S}{\omega_{50}} \right)$$

- ✦ Gravel equation: (D_{50} between 2mm and 10mm)

$$\log C_w = 6.681 - 0.633 \log \frac{\omega_{50} D_{50}}{\nu} - 4.816 \log \frac{V_*}{\omega_{50}} + \left(2.784 - 0.305 \log \frac{\omega_{50} D_{50}}{\nu} - 0.282 \log \frac{V_*}{\omega_{50}} \right) \log \left(\frac{v_{ch} S}{\omega_{50}} - \frac{V_{cr} S}{\omega_{50}} \right)$$



Selecting the appropriate model

Model	Gravel	Sand	Very Fine sand and silt
Bagnold		X	X
Kodatie	X	X	X
Molinas and Wu		X	
Yangs	X	X	



Deposition

- ✦ If the sediment concentration in the channel is more than the transport capacity then deposition occurs:
- ✦ Einstein equation (1965):

$$Dep_{fract} = \left(1 - \frac{1}{e^x} \right)$$

$$x = \frac{1.055 \cdot L_{ch} \cdot \omega_{50}}{v_{ch} \cdot depth}$$

$$\omega_{50} = \frac{411 \cdot D_{50}^2}{3600}$$

- ✦ Flood plain deposition
 - ✦ If the streamflow goes overbank



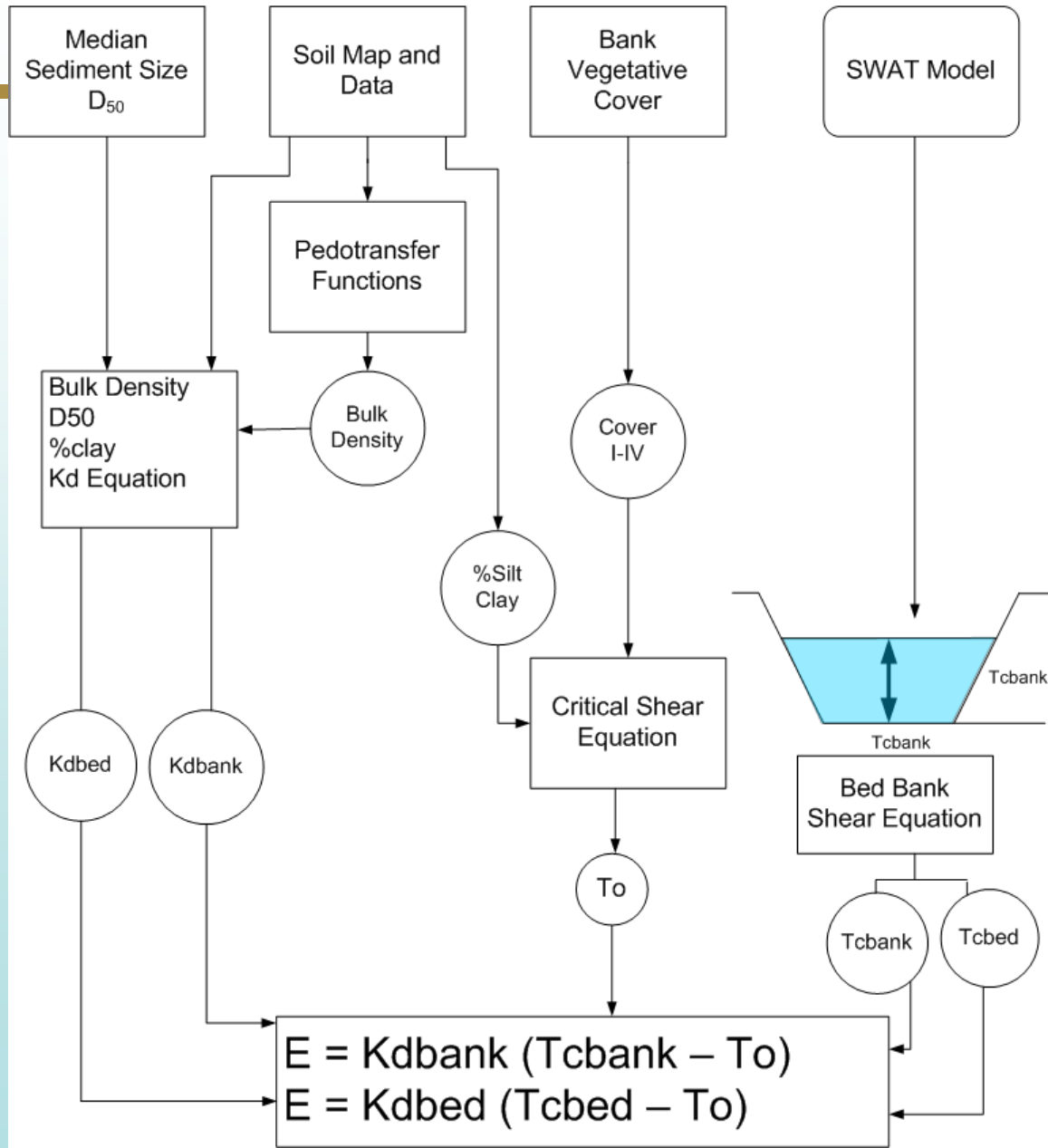
Excess transport capacity

$$SedEx = V_{ch} \cdot (conc_{sed,ch.mx} - conc_{sed,ch.i})$$

- ✦ Excess sediment beyond transport capacity is also deposited
- ✦ But the channel is eroded only based on excess shear stress and not the available transport capacity
 - ❏ Bank scour always occurs when excess shear stress is available
 - ❏ Bed scour occurs only after all the deposited bed materials are scoured



Channel Erosion Procedure





Study Area

📍 Kings Creek Watershed

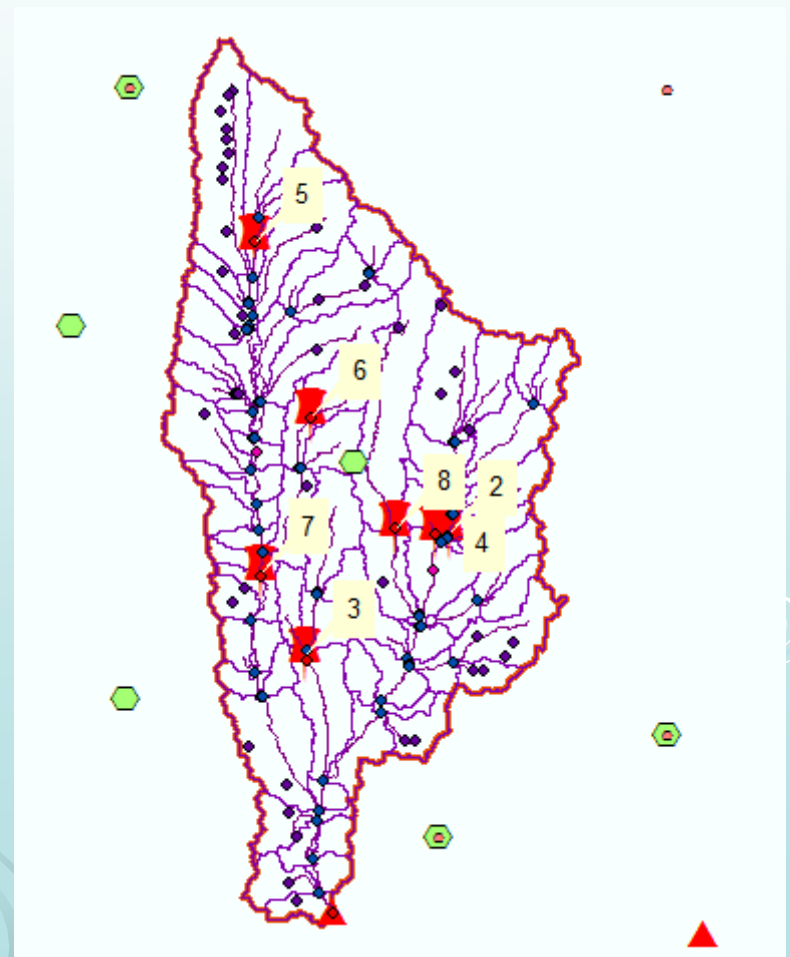




Figure 8. Photograph of the upper and lower bank erosion pin locations shown in yellow, spaced 1 m apart after Zaimes et al. 2005.

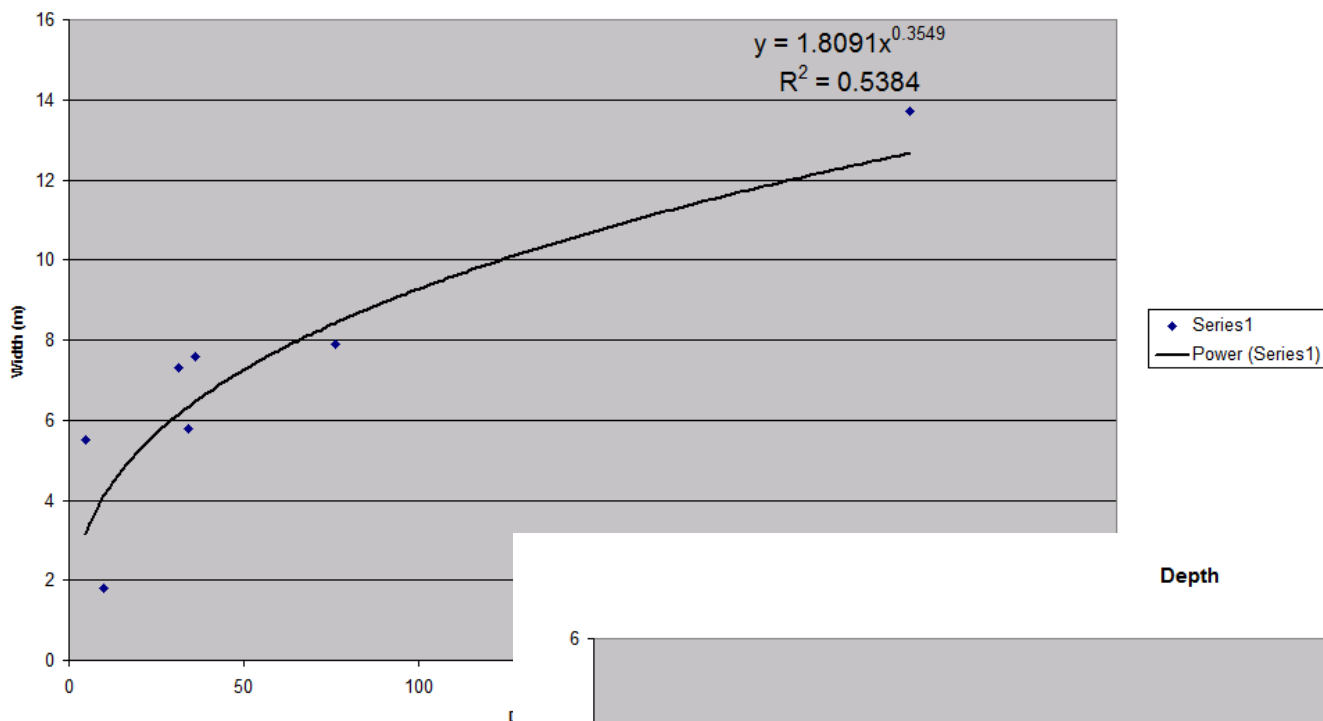


Field Data Collection

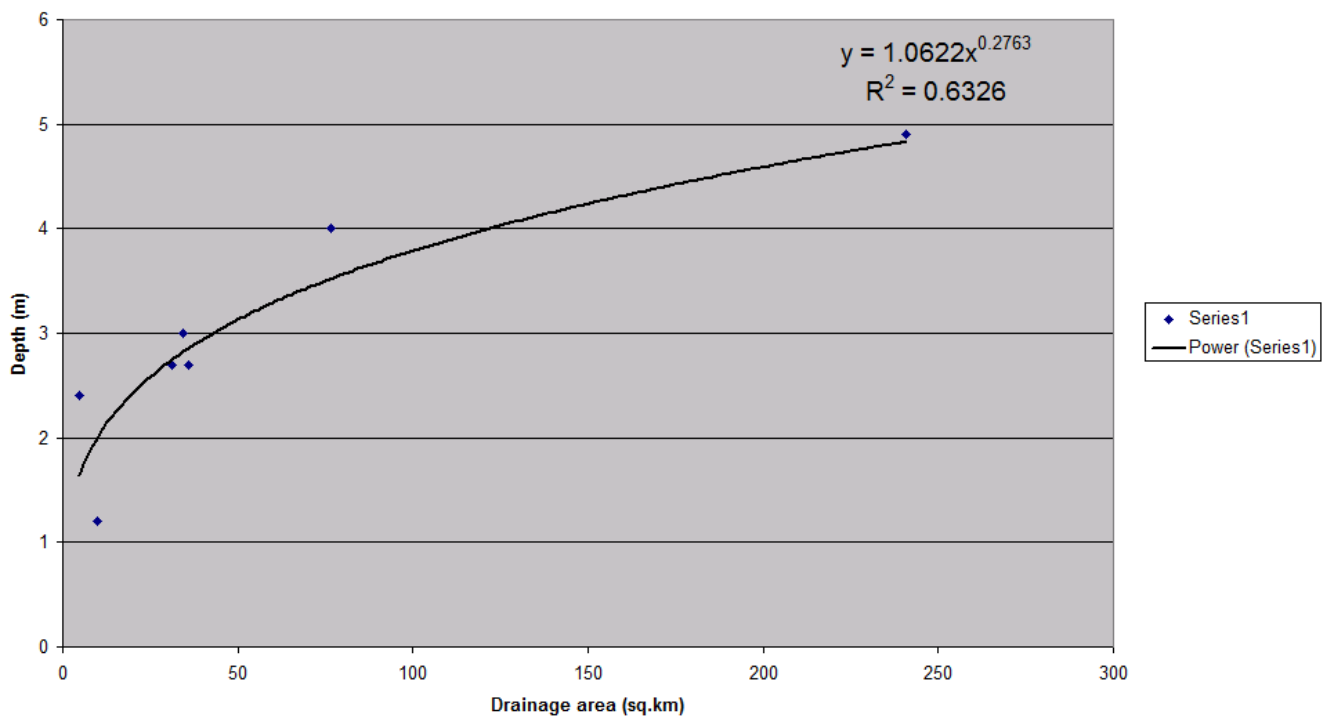
- ⊕ Stage height
- ⊕ Erosion rate (mm/event)
- ⊕ Channel dimension
- ⊕ Particle size distribution
- ⊕ Submerged jet test
 - ⊕ **Erodibility**
- ⊕ Period: 2007

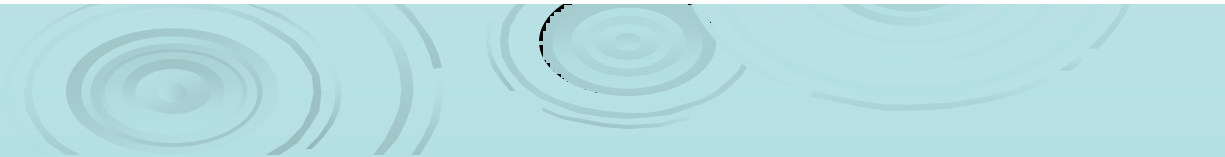
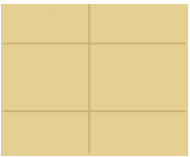
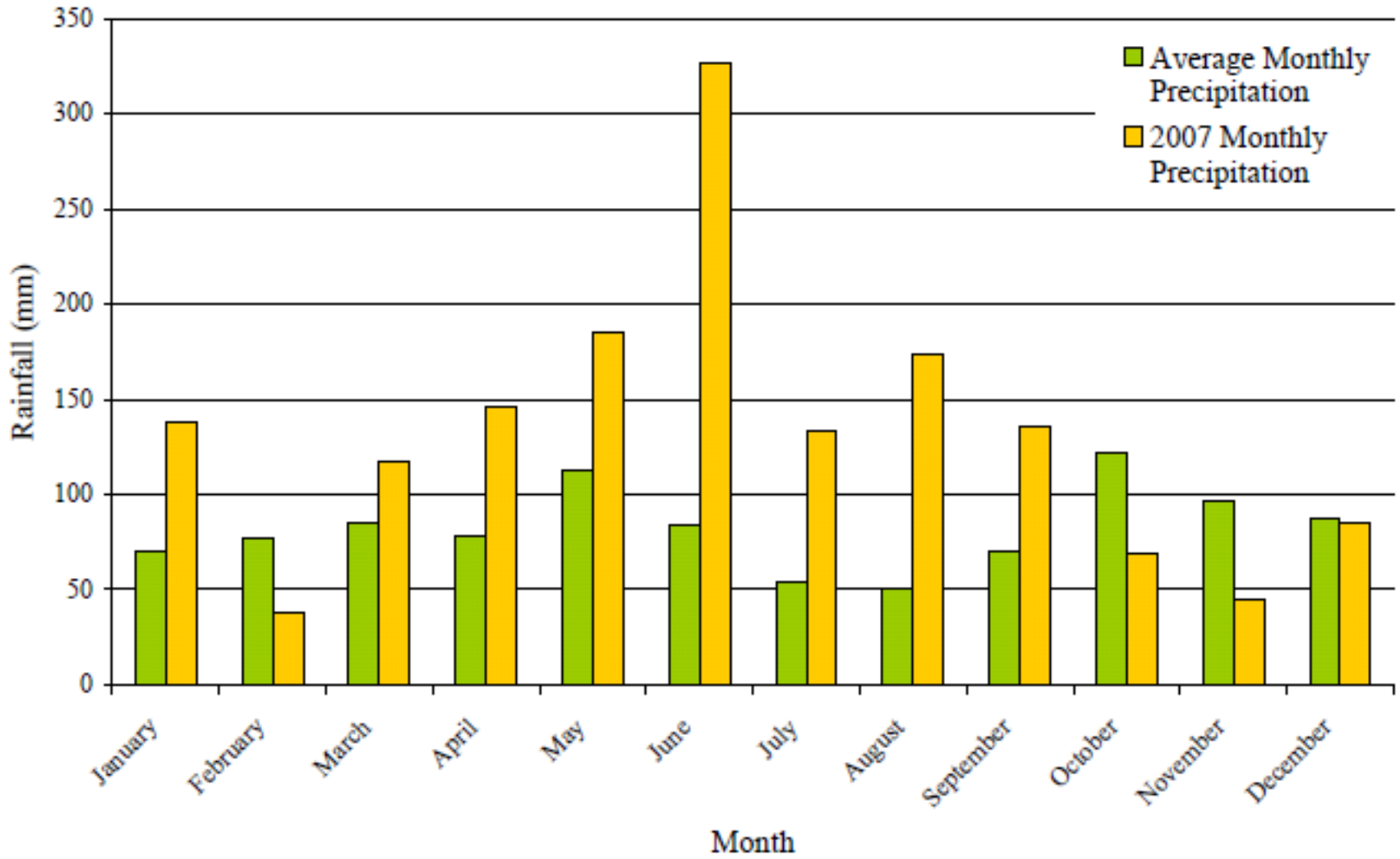


Width



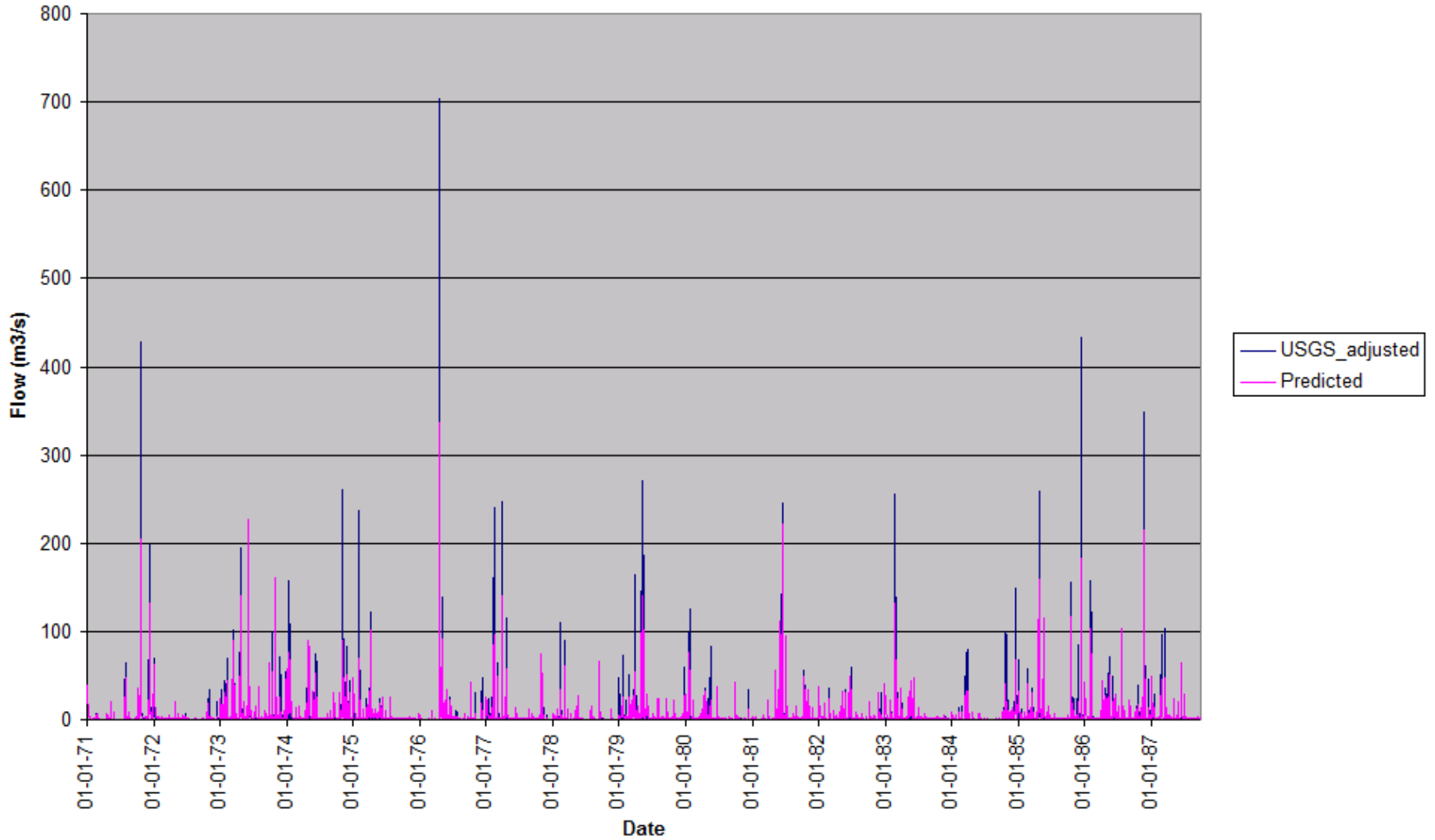
Depth

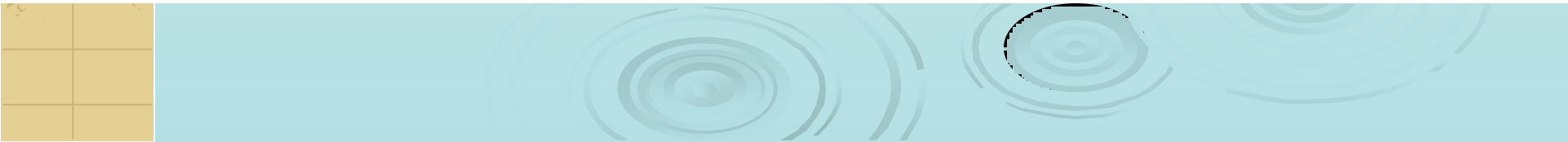
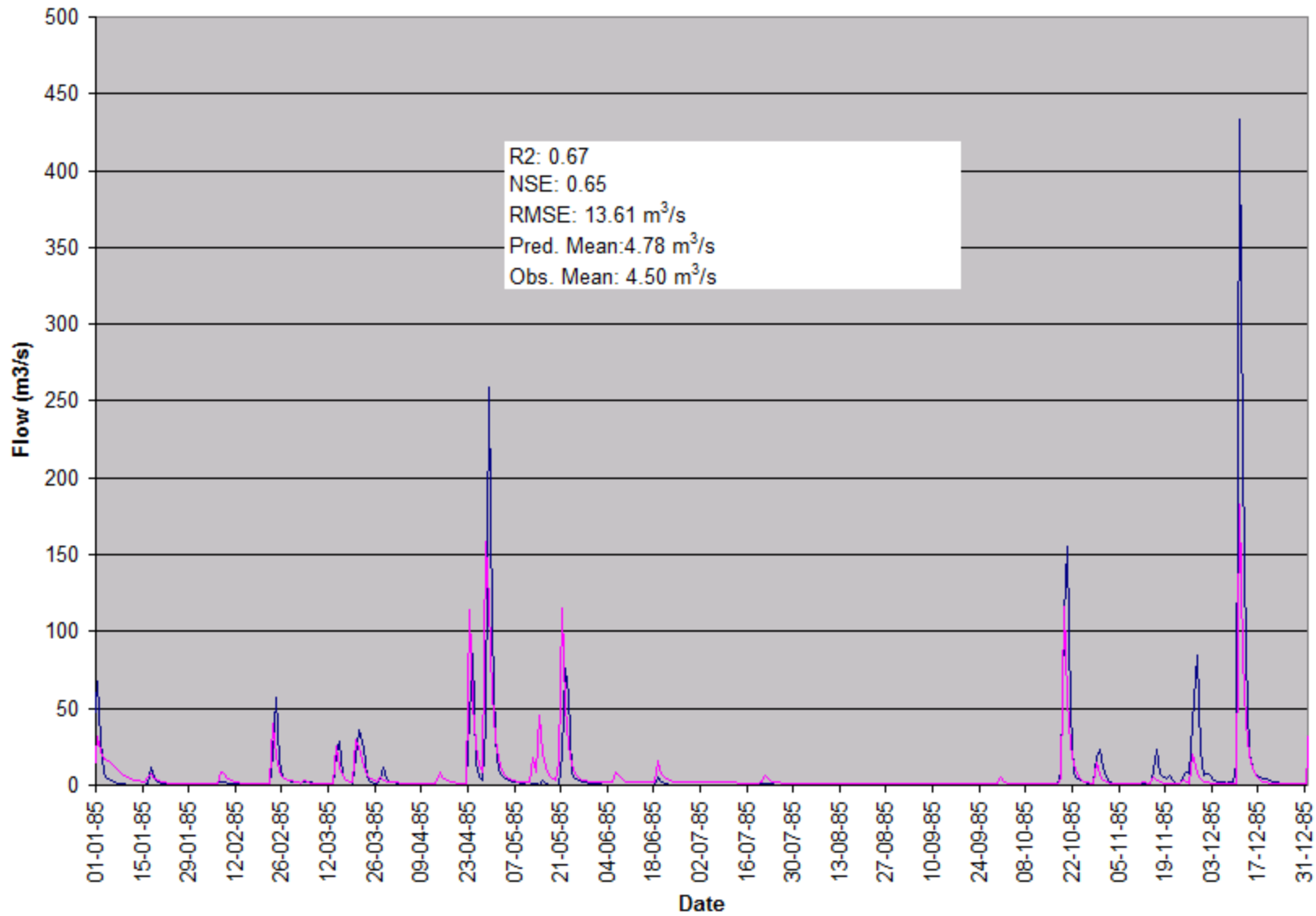






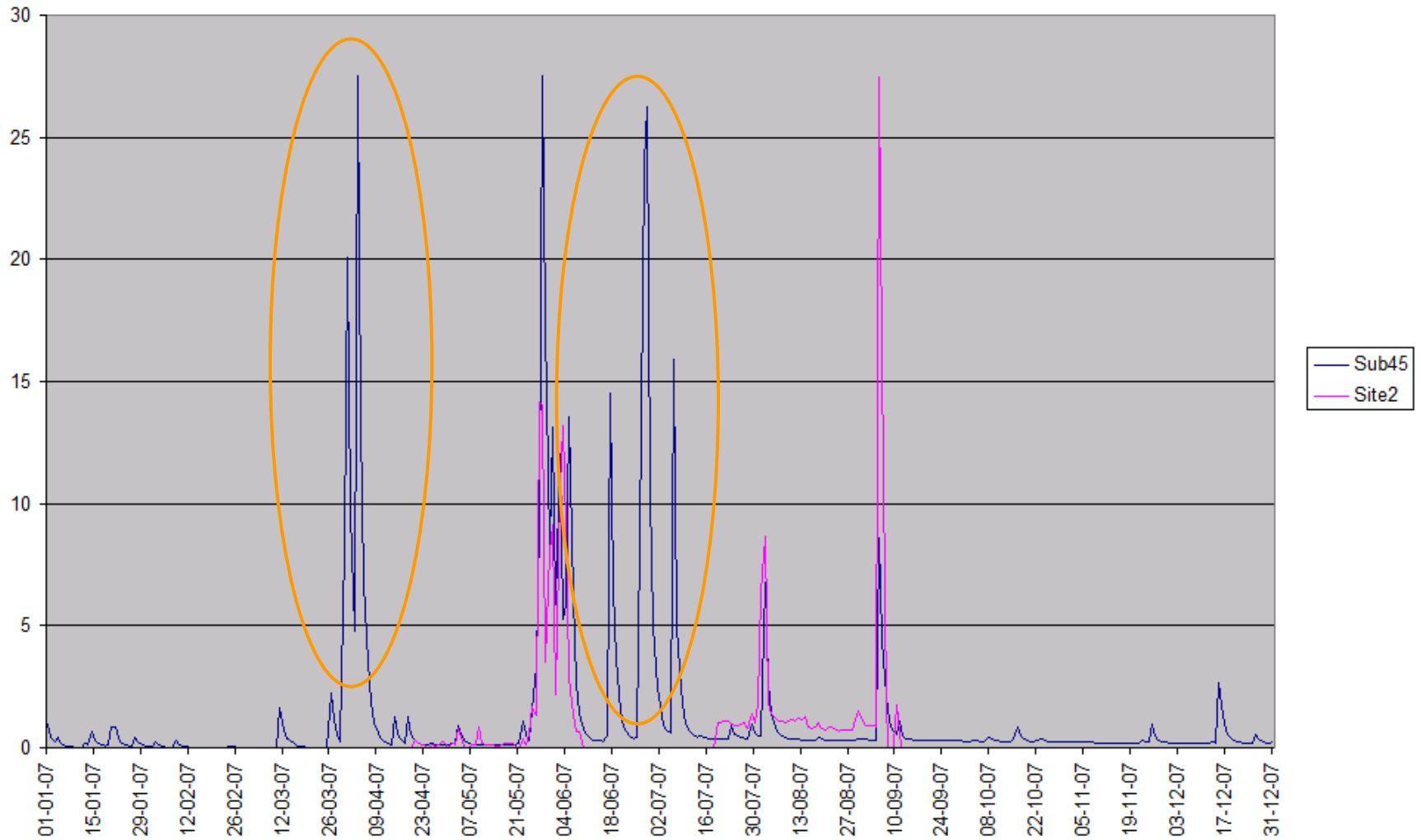
Flow Calibration (1971-1987)





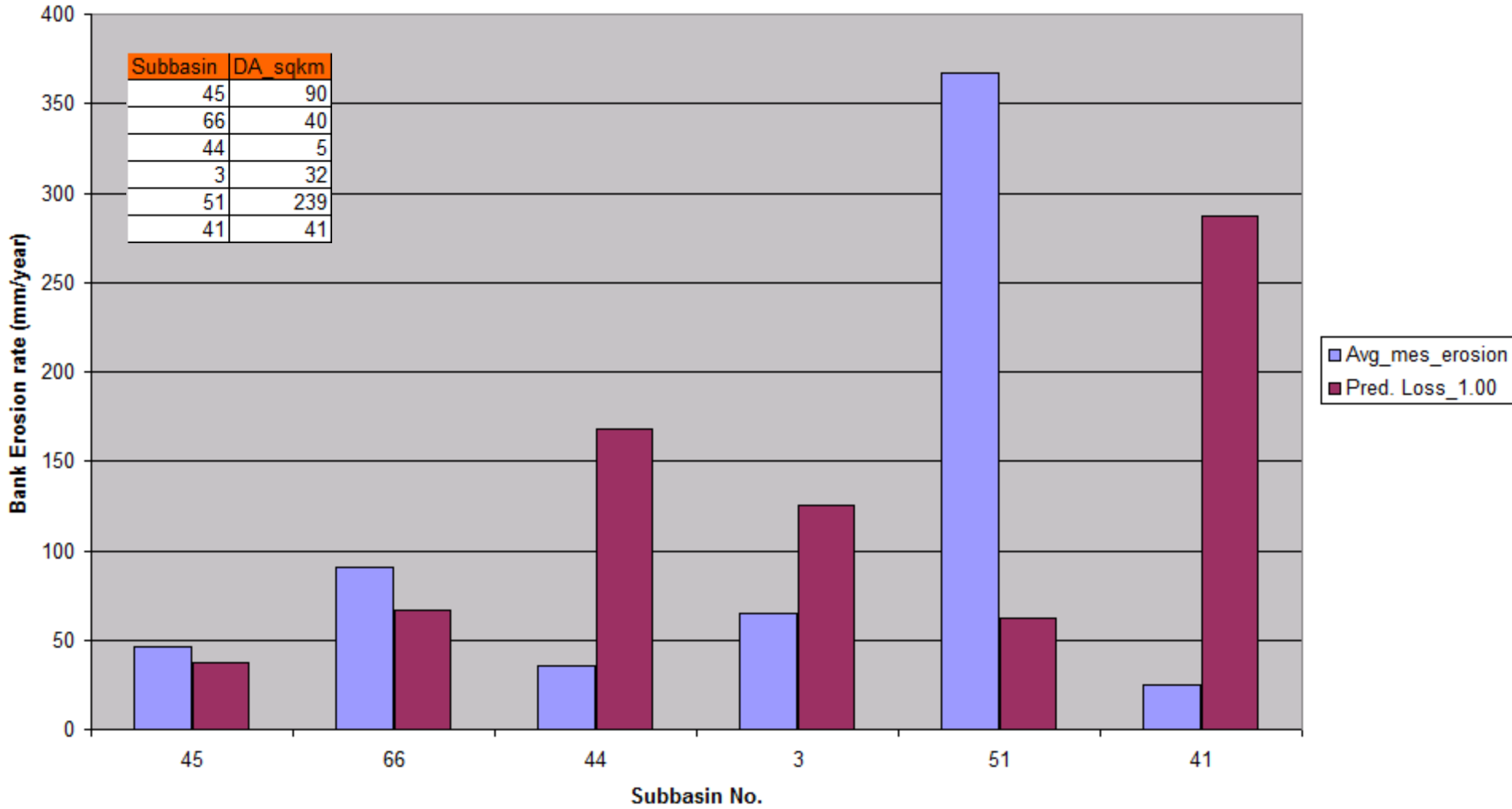


Flow Validation (2007)

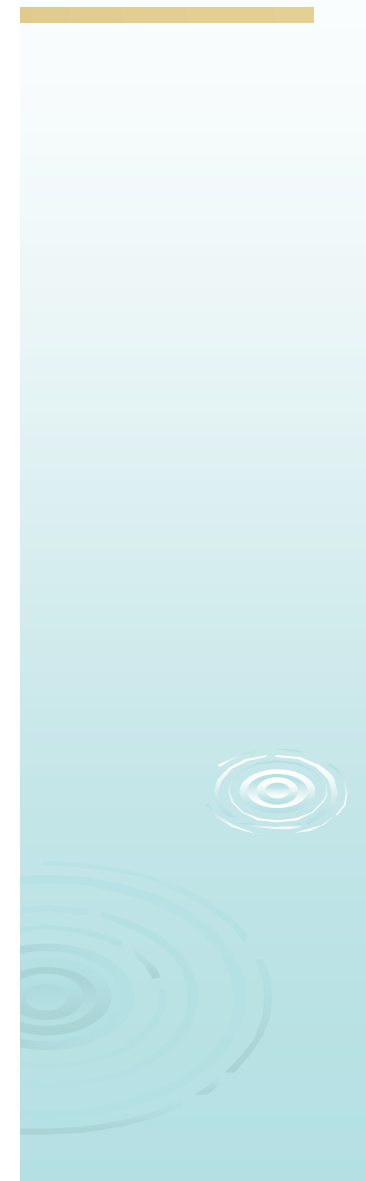
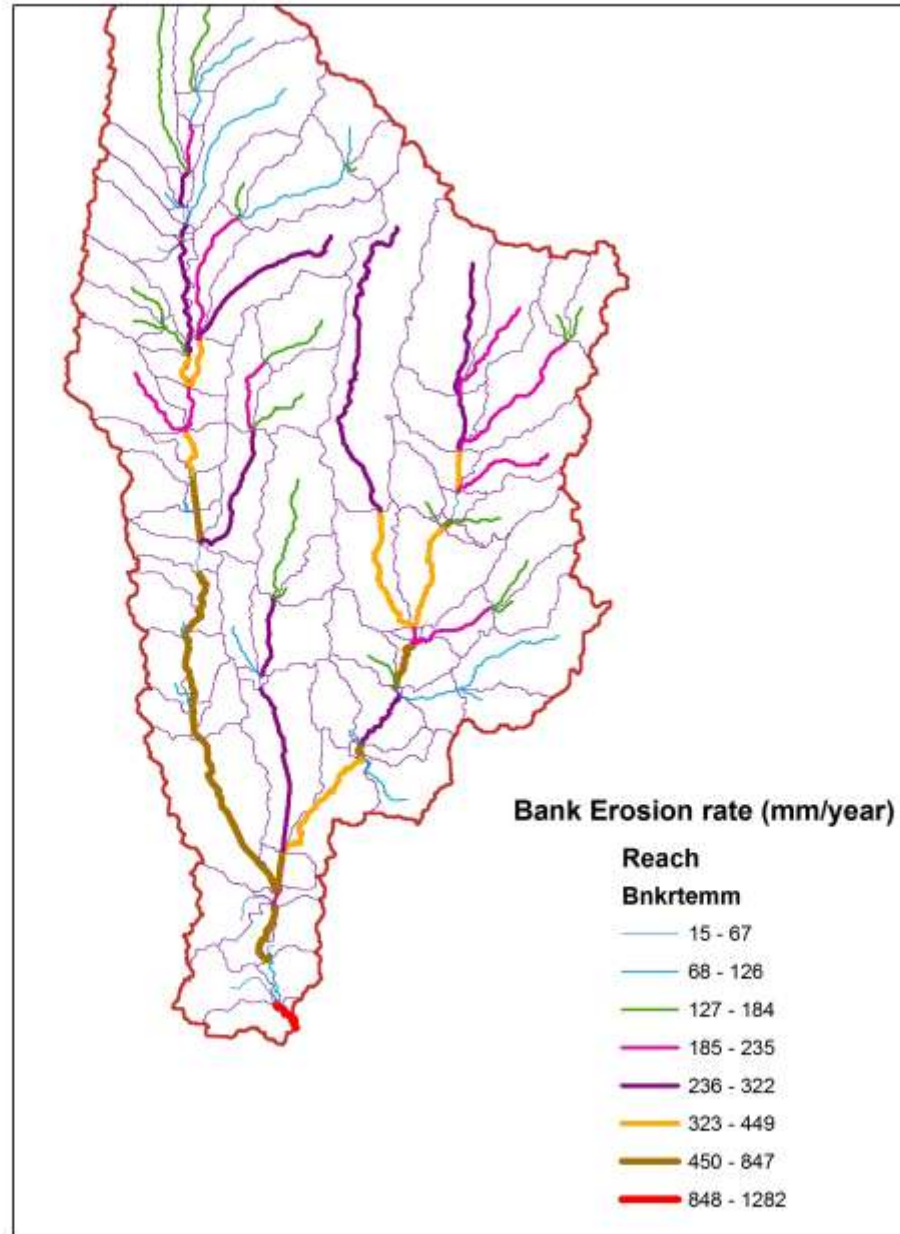
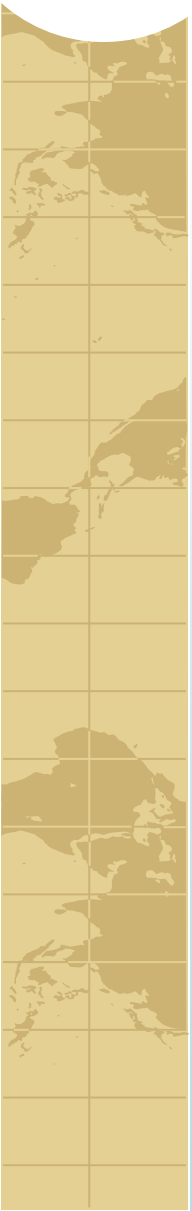


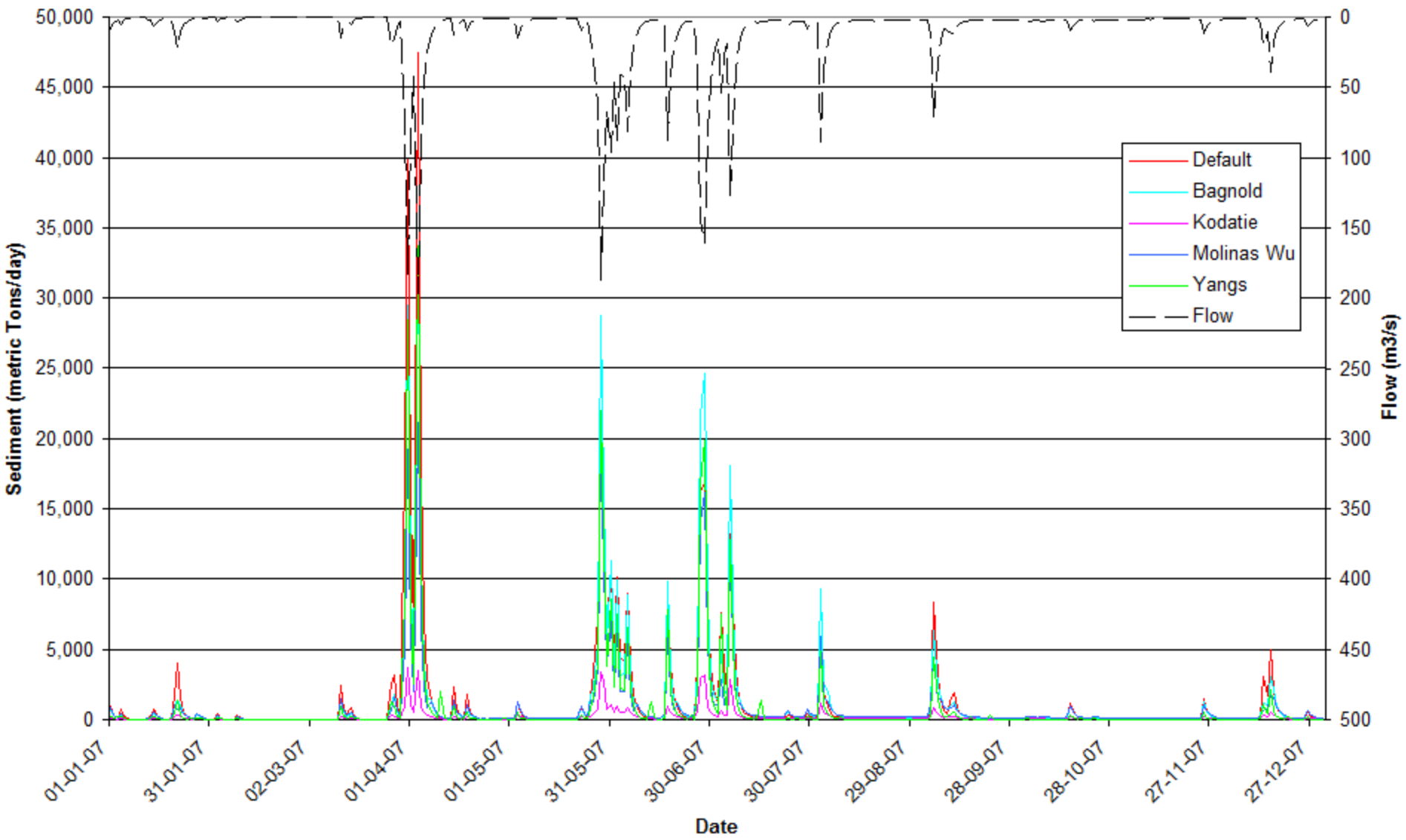


2007 Bank Erosion Rate



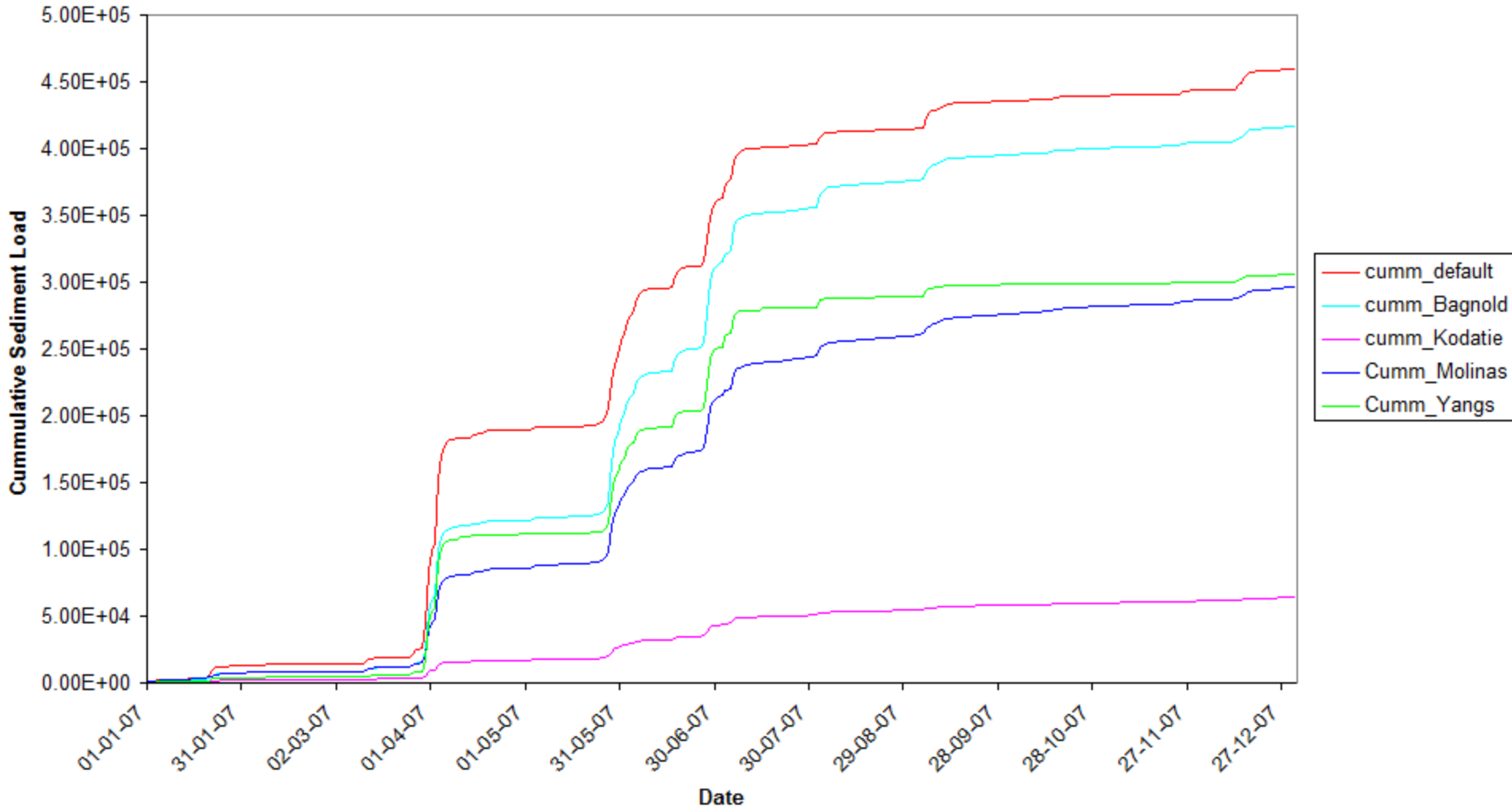
Subbasin	DA_sqkm
45	90
66	40
44	5
3	32
51	239
41	41

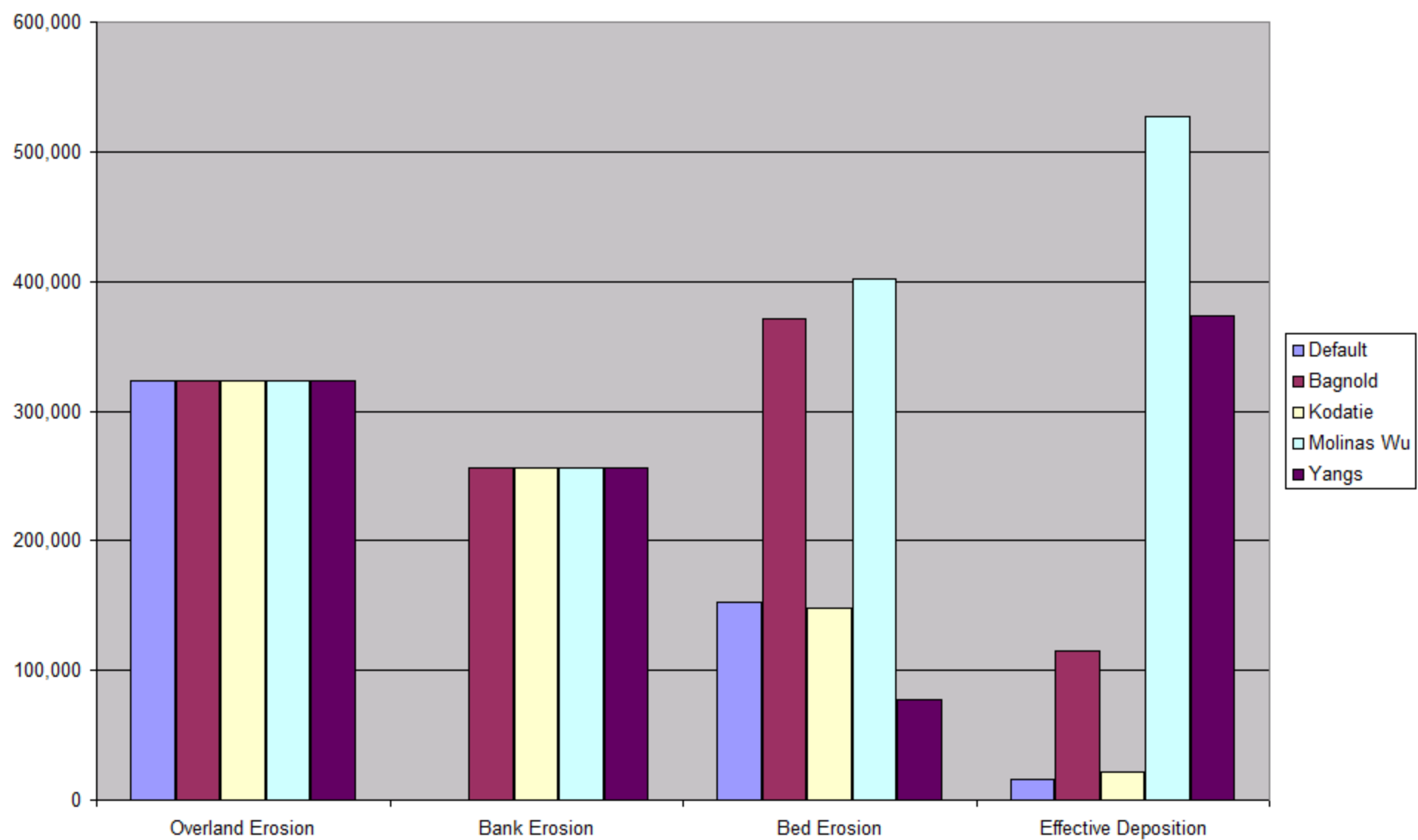






Cummulative Sediment Load







Model Inputs

✦ Default model

- ✦ spcon, spexp, CH_cov, CH_Erod

✦ Physically based models

- ✦ D50 – Median particle size of bank and bed material
- ✦ Cover factor of bank and bed
- ✦ Critical shear stress of bank and bed
 - If not given, calculated based on SC% and cover
- ✦ Erodibility coefficient of bank and bed
 - If not given, calculated based on SC%
- ✦ Bulk density of bank and bed
 - If not given, calculated based on SC%
- ✦ Particle size distribution of bank and bed material
 - Assumed based on the D50 size



Model Output

- ⊕ File name: output.sed

- ⊕ Default

 - ⊕ Total sediment

 - ⊕ Bed erosion, deposition, TSS

- ⊕ Physically based models

 - ⊕ Total sediment

 - Sand, silt, clay, SAGG, LAGG, gravel

 - ⊕ Bank erosion

 - ⊕ Bed erosion

 - ⊕ Channel deposition / Flood plain deposition

 - Total remaining in deposits at the end of the time step

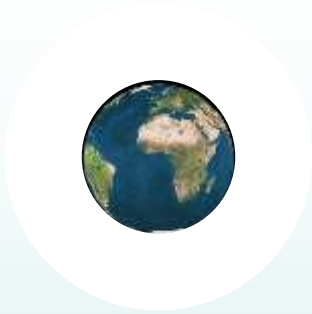
 - ⊕ TSS

 - Only based on silt and clay particles



Conclusion

- ➊ Already most of the code is available in the present release
- ➋ Few changes are being made to represent the mass balance in a better way
- ➌ Detailed calibration and validation study is underway to evaluate the new routines
- ➍ **New Components**
 - ❏ Active channel eroding length based on channel sinuosity
 - ❏ Effect of alternate wetting and drying on erodibility



Thank You

